



## REVISED REPORT OF GEOTECHNICAL EXPLORATION

New Nicollet Redevelopment Nicollet Ave between Cecil Newman Ln and Lake St Minneapolis, Minnesota

AET Project No. P-0045582

Date: November 5, 2025

## Prepared for:

Minneapolis Department of Community Planning and Economic Development 505 4th Avenue South, Suite 320 Minneapolis, Minnesota 55415

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November 5, 2025

Minneapolis Department of Community Planning and Economic Development 505 4th Avenue South, Suite 320 Minneapolis, Minnesota 55415

Attn: Linnea Graffunder-Bartels

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RE: Revised Report of Geotechnical Exploration

New Nicollet Redevelopment

Nicollet Ave between Cecil Newman Ln and Lake St

Minneapolis, Minnesota AET Report No. P-0045582

Dear Mrs. Graffunder-Bartels:

American Engineering Testing, Inc. (AET) is pleased to present the results of our subsurface exploration program and geotechnical engineering review for your New Nicollet Redevelopment project in Minneapolis, Minnesota. These services were performed according to our proposal to you dated July 24, 2025.

We are submitting an electronic (pdf) copy of the report to you.

Please contact me if you have any questions about the report. I can also be contacted for arranging construction observation and testing services.

Sincerely,

American Engineering Testing, Inc.

Reed Kaeppe, PE (MN, IA, WI)

**Engineer II** 

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## SIGNATURE PAGE

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Engineer II

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under Minnesota Statute Section 326.02 to 326.15

Name: Reed Kaeppe

Date: November 5, 2025

License #: 60080

Reviewed by:

Thomas Evans, PE (MN)

Senior Engineer



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## 1.0 INTRODUCTION

The Minneapolis Department of Economic Planning and Development envisions two new apartment buildings in Minneapolis, Minnesota, according to the New Nicollet Development Framework approved by the Mayor and City Council in May 2025. To assist planning and design, you have authorized American Engineering Testing, Inc. (AET) to conduct a subsurface exploration program at the site, conduct soil laboratory testing, and perform a geotechnical engineering review for the project. This report presents the results of the above services and provides our engineering recommendations based on this data.

#### 2.0 SCOPE OF SERVICES

AET's services were performed according to our proposal to you dated July 24, 2025, which you authorized on August 4, 2025. The authorized scope consists of the following.

- Drilling 10 Standard Penetration Test (SPT) borings including the following:
  - 4 SPT borings to a depth of 35 feet within the proposed building areas.
  - 4 SPT borings to a depth of 20 feet within the proposed building areas.
  - o 2 SPT borings to a depth of 5 feet within the proposed public park.
- Performing requested soil laboratory testing.
- Geotechnical engineering review based on the data and preparation of this report.

These services are intended for geotechnical purposes only. AET also performed a Phase I Environmental Site Assessment (ESA) and a Phase II ESA. Our findings and recommendations will be submitted under separate covers.

## 3.0 PROJECT INFORMATION

We understand the Minneapolis Department of Economic Planning and Development is proposing the reconstruction of Nicollet Ave between the bridge over the Midtown Greenway and Lake St in Minneapolis, Minnesota, as well as several residential and mixed use structures on the east side of the reconstructed Nicollet Avenue. Future development on the west side of Nicollet Avenue was not included in this project scope. The proposed facilities consist of a public park, one 5 story residential structure, one 4 story residential structure, constructing Nicollet Avenue, concrete paved sidewalks, and patio spaces. We understand the structures will have one story of below grade parking. Additionally, we understand that the first two stories in each of the structures will be comprised of precast panels, while the remaining stories will consist of wood framing.



Our foundation design assumptions include a minimum factor of safety of 3 with respect to the ultimate bearing capacity. We assume the structures each will be able to tolerate total settlements of up to 1 inch, and differential settlements over a 30-foot distance of up to ½ inch.

The above stated information represents our understanding of the proposed construction. This information is an integral part of our engineering review. It is important that you contact us if there are changes from that described so that we can evaluate whether modifications to our recommendations are appropriate.

### 4.0 SUBSURFACE EXPLORATION AND TESTING

#### 4.1 Field Exploration Program

The subsurface exploration program conducted for the project consisted of 10 standard penetration test borings performed on September 3<sup>rd</sup>, 4<sup>th</sup>, and 5<sup>th</sup>. AET determined the number, depth, and locations of the soil borings.

The borings were located in the field by AET personnel using a GPS unit with sub-meter accuracy. Surface elevations were estimated using publicly available topographic data furnished by MnTOPO (<a href="http://arcgis.dnr.state.mn.us/maps/mntopo/">http://arcgis.dnr.state.mn.us/maps/mntopo/</a>). The ground surface elevations at the drilled locations are shown on top of the boring logs. The ground surface elevations shown on the logs are approximate and should not be used for the project design or construction.

The logs of the borings and details of the methods used appear in Appendix A. The logs contain information concerning soil layering, soil classification, geologic origins, and moisture condition. A density description or consistency is also noted for the natural soils, which is based on the standard penetration resistance (N-value).

## 4.2 Laboratory Testing

The laboratory test program included visual/manual classification of the soil samples, water content tests, sieve analyses, and percent passing the #200 sieves. The moisture contents and percent passing the #200 sieve values are shown on the subsurface boring logs adjacent to the samples upon which they were performed. The full gradation curves are attached in the appendix following the boring logs.



## 5.0 SITE CONDITIONS

#### **5.1 Surface Observations**

The proposed development is located south of the intersection of Nicollet Ave and the Midtown Greenway in Minneapolis, Minnesota. The lot is developed with bituminous pavements associated with a previously demolished structure. The elevation on site is relatively flat. The surface elevations at the boring locations ranged from 881 feet at Boring B-6 to 885 feet at Boring B-9.

### 5.2 Subsurface Soils/Geology

The site geology consists of fill soils to depths of up to 14½ feet underlain by coarse alluvium to the termination depths of the borings at 6 to 36½ feet below grade. The fill soils consist of sandy lean clay, clayey sands, silty sands, sands with silt, and sands.

The underlying coarse alluvium consists of loose to very dense sands with variable gravel content. The N-values in these soils ranged from 6 to 54 blows per foot. Please refer to the boring logs for additional information.

#### 5.3 Groundwater

Groundwater was not measured or observed in any of the borings. In the borings, the encountered soils were predominantly fast draining sandy coarse alluvial soils. In these soils, it may take hours for a water level to develop and reach equilibrium in an open borehole. Therefore, groundwater may be encountered at different depths than in our borings, or in areas beyond the extent of our exploration.

More accurate groundwater levels can be obtained by installing and monitoring piezometers; however, this was not part of our work scope. Groundwater levels do not remain static and fluctuate due to varying seasonal conditions and annual rainfall and snow melt amounts, as well as other factors.

## 5.4 Review of Soil Properties

#### 5.4.1 Fill or Possible Fill

In our opinion, the fill soils are low strength materials and are judged to be potentially compressible under structure loads or where grades are raised. The fill is generally judged to be moderately fast draining and has moderate frost susceptibility.



#### 5.4.3 Coarse Alluvium

The coarse alluvial soils are judged to have moderately high to high strength and low compressibility. They are fast draining and have moderately to low frost susceptibility when they are impacted by freezing temperatures.

## 6.0 RECOMMENDATIONS

#### 6.1 Discussion

Fill and possible fill soils were encountered to depths of up to 14½ feet below grade at our boring locations; however, fill soil depths can and will vary between boring locations. The N-values of the fill soils ranged from 4 to 39, indicating an extremely variable soil layer. Additionally, debris and trace organics were encountered at several boring locations.

It is our opinion the existing fill soils should not be left in place to support the proposed structures. Relying on the existing fill soils could lead to unacceptable total and differential settlements. We recommend removing the existing fill to the naturally deposited coarse alluvial soils and placing engineered fill in compacted lifts to design foundation and/or slab elevations.

#### 6.2 Excavation

To prepare the structures for foundation and floor slab support, we recommend complete excavation of the fill soils, thereby exposing the underlying alluvial sandy soils. This would result in excavation depths at the boring locations as shown in Table 6.2.

Table 6.2 – Recommended Excavation Depths

Boring Number	Surface Elevation (ft) <sup>A</sup>	Excavation Depth (ft, below existing grade) <sup>B</sup>	Approximate Excavation Elevation (ft) <sup>B</sup>
B-01	884	7-9	877-875 <sup>c</sup>
B-02	882	7	875
B-03	885	4½	880½
B-04	882	7	875
B-05	883	4½	878½
B-06	881	3	878
B-07	884	12	872
B-08	884	14½	869½

Notes: A -Rounded to the nearest tenth of a foot.

B - Rounded to the nearest ½ foot.

C – Soils classified as "Alluvium or Fill" should be observed at the time of construction to determine geologic origin. Deeper excavation depth/elevation applies if determined to be undocumented fill soil.



The estimated excavation depths in Table 6.2 are based on the soil conditions at the specific boring locations. These depths will need to be evaluated during construction. Because conditions will vary away from the boring locations, we recommend that AET geotechnical personnel observe and evaluate the competency of the soils in the excavation bottom prior to new fill or footing placement. If soft or wet soils are encountered at the base of the excavation, they should be subcut and replaced or moisture conditioned and recompacted.

If the excavation extends below foundation grade, the excavation bottom must be oversized laterally beyond the planned outside edges of the foundations to properly support the lateral loads exerted by that foundation. This excavation oversizing should equal the vertical depth of fill needed to re-attain foundation grade at that location (i.e., 1H:1V oversizing).

Prior to fill placement or foundation construction, the exposed sands in the excavation bottom should be surface compacted with a vibratory compactor. Surface compaction should involve at least 6 passes with a vibratory roller compactor having a minimum static weight of 10 tons. The compactor passes should be performed in perpendicular directions (e.g., 3 passes east-west and 3 passes north-south). After surface compaction has been performed, refilling to attain footing grade may begin.

Groundwater was not observed at or above termination depths within the borings. Therefore, we do not expect that groundwater will be encountered within the required building excavations. If groundwater is encountered within the excavations, it should be removed. Dewatering means and methods are the responsibility of the contractor.

## **6.3 Fill Placement and Compaction**

Within the apartment building footprints, the on-site, non-organic, debris-free soils can be used as structural backfill. It is beyond our scope of services to estimate the volume of soils that can be reused onsite. If additional fill is needed, it should consist of soils substantially matching the existing soils in moisture content and composition, such as sands with a total fines content (percent passing the #200 sieve) of less than 12%. If the contractor proposes a different type of fill, a sample should be submitted to the Geotechnical Engineer for approval.

Fill placed to attain grade for foundation support should be compacted in thin lifts, such that the entire lift achieves a minimum compaction level of 98% of the standard maximum dry unit weight per ASTM: D698 (Standard Proctor test). Fill soils should be moisture conditioned (e.g. wetted or dried) to within ±2% of their optimum moisture content per the standard Proctor test.



All fill should be free of debris, rubble, organics, and other unsuitable materials. All fill soils should be compacted with equipment which will densify the entire lift of fill. Fill should not be placed over frozen soils, and frozen soils should not be used as fill.

If earthwork operations take place during freezing weather, all frozen soils, snow, and ice should be stripped from the areas to be filled prior to new fill placement. The new fill should not be allowed to freeze during transit, placement, or compaction. We refer you to the attached sheet entitled "Freezing Weather Effects on Building Construction" for additional information.

#### 6.4 Foundations

It is our opinion that the proposed apartment buildings can be supported on conventional spread footings following the above recommended earthwork. The new building foundations would be supported on the naturally-deposited alluvial silty sands or on the newly placed and compacted engineered fill. We recommend perimeter foundations for heated building spaces be placed such that the bottoms are a minimum of 42 inches below exterior grade. We recommend foundations for the unheated structures be extended to a minimum of 60 inches below exterior grade. For buildings with below grade basements, the foundations may be placed directly below the grade beams or basement walls.

Based on the conditions encountered, it is our opinion the building foundations can be designed based on a net maximum allowable soil bearing pressure of 4,000 psf. This refers to the pressure that may be transmitted to the bearing stratum in excess of the pressure from the surrounding depth of overburden. The footings should have a minimum width of 24 inches for strip footings and 36 inches for column footings to avoid disproportionally small footing sizes. It is our judgment this design pressure will have a factor of safety of at least 3 against the ultimate bearing capacity. We estimate that total settlements under this loading should not exceed 1 inch. We also estimate that differential settlements of conditions depicted by the borings should not exceed ½ inch.

The bottoms of all foundation excavations should be free of water and loose soil prior to placing structural fill or concrete. Structural fill should be placed soon after excavating to reduce bearing soil disturbance, and concrete should be placed soon after excavating or completion of the structural fill placement. If the materials at bearing level become excessively dry, disturbed, saturated, or frozen, the affected material should be removed and replaced prior to placing concrete.

## 6.5 Floor Slab Design

All fill soils that are placed below on-grade floor slabs, including foundation backfill and in underslab utility trenches, should be compacted to at least 95% of the Standard Proctor



maximum dry density. The fill soils should be placed in loose lift thicknesses not exceeding ten inches to allow compaction of the entire layer with the equipment being used.

The on-grade slabs can be supported with the in-situ non-organic fill or alluvial soils. We recommend using a modulus of subgrade reaction (k-value) of 200 psi/inch for these soils. Alternatively, the floor slab can be supported on 6 inches of imported aggregate material, meeting the gradation requirements of MnDOT Class 5 (Table 3138.2-3). If a 6-inch thick layer of Class 5 is used, an increased modulus of 230 psi/inch can be used.

For recommendations pertaining to moisture and vapor protection of interior floor slabs, we refer you to the standard sheet entitled "Floor Slab Moisture/Vapor Protection" at the end of this report. A vapor retarder should be placed under the floor slab where there are moisture sensitive floor coverings/coatings.

#### 6.6 Exterior Building Backfill

Fill that is placed below sidewalks, stoops, and exterior slabs should be compacted to a minimum of 95% of the standard Proctor maximum dry density. In the upper 3 feet of pavement subgrades, compaction should be increased to 100%. Fill placed in landscaped areas should be compacted to a minimum of 90% of the standard Proctor maximum dry density.

For design considerations that could help mitigate problems related to frozen soil behavior we refer you to the attached sheet entitled "Freezing Weather Effects on Building Construction".

#### 6.7 Basement Backfill

The walls associated with the basements will act as cantilever retaining walls. We recommend the walls be backfilled with free-draining sand having less than 5% passing the No. 200 sieve and no more than 40% of the particles (by weight) finer than the No. 40 sieve. The zone of sand backfill should extend outward from the wall at least 2 feet, and then upward and outward from the wall at a 30 degree or greater angle from vertical. The sand backfill should be placed in lifts and compacted with portable compaction equipment to at least 95% of the maximum Standard Proctor dry density (ASTM D698). For the sand backfill previously described, we recommend the following ultimate lateral earth pressure values (given in equivalent fluid pressure values), as well as other soil parameters be used in design of the retaining walls:



Table 6.7 - Soil Parameters for Sand Backfill

Active Earth Pressure	35 psf/ft
Passive Earth Pressure	410 psf/ft
At-rest Earth Pressure	60 psf/ft
Internal Friction Angle	32 degrees
Sliding Friction Coefficient	0.35
Unit Weight	125 pcf

Because movement is required to develop the full passive pressure, we recommend applying a factor of safety of at least 2 to the above passive value for design. The lateral earth pressures are not applicable for submerged soils/hydrostatic loading and do not include surcharge loading.

Compaction within 5 feet of the back of the walls should be accomplished with hand-operated tampers or other lightweight compactors. Over-compaction may cause excessive lateral earth pressures which could result in unexpected wall movement or cracking. Caution should be exercised about vibration effects on nearby structures from soil compaction activities. In highly vibration-sensitive areas, backfill with 3/8" minus pea rock, self-compacting, wrapped in fabric to prevent the migration of fines.

Please refer to the attached standard sheet entitled "Basement/Retaining Wall Backfill and Water Control" for additional information on our recommendations on backfilling basement walls.

## 7.0 UTILITY RECOMMENDATIONS

## 7.1 Utility Installation Discussion

The depths of the proposed utilities have not been provided. However, we assume they will be placed at depths of 12 feet or less. Based on the conditions encountered at the borings, these utilities will likely bear within the coarse alluvial sands.

Groundwater was not observed in any of the borings below grade. In the borings, the encountered soils were predominantly fast draining sandy coarse alluvial soils. In these soils, it may take hours for a water level to develop and reach equilibrium in an open borehole. Therefore, groundwater may be encountered at different depths than in our borings, or in areas beyond the extent of our exploration. Due to the relatively high permeability of the site soils, sump pumps may not be sufficient to dewater the site if active groundwater conditions are encountered during construction. Instead, more intricate dewatering methods, such as well points, may be needed. Dewatering means and methods should be determined by the construction team based on the conditions encountered during excavation.



### 7.2 Utility Support

Based on the conditions encountered at the boring locations, it is our opinion that the soils exposed in utility excavations at this site should generally be suitable for utility support. Gravel was observed within the alluvial layers at several boring locations. Therefore, we recommend providing a 4 to 6-inch thick layer of Granular Bedding (MnDOT Spec. 3149.2F) directly beneath the pipes, per the pipe manufacturers recommendations. The bedding should be shaped to conform to the bottom of the pipe to minimize point or imbalanced loads on the pipe and provide uniform pipe support.

If unstable soils are encountered and additional sub-cutting is necessary to provide pipe support, the excavation for pipe foundation improvement should be laterally oversized at the bottom a horizontal distance (from the outermost plan view point of the pipe) at least equal to the vertical distance between the lowest bottom elevation of the pipe and the lowest excavation bottom elevation (i.e., 1H:1V lateral oversize).

#### 7.3 Backfill and Compaction

On-site, inorganic, soils may be suitable for reuse as utility backfill provided they can be properly moisture conditioned and compacted. The fill soils should be free of organic matter, rubble, debris, or gravel larger than 3 inches in the largest dimension. Utility trench backfill soils should match the adjacent subgrade soils when placed within 3 feet of grading grade.

Utility backfill soils should be placed in lift thicknesses appropriate to the compaction equipment being used and the soil being compacted. The compactor should be capable of compacting the entire lift thickness to the recommended compaction level. We recommend trench backfill placed within 3 feet of grading grade be compacted to a minimum of 100% of the Standard Proctor (ASTM D698) maximum dry unit weight. The remaining utility trench backfill below the upper 3 feet can have a minimum compaction level of 95% of the Standard Proctor maximum dry unit weight.

## 8.0 CONSTRUCTION CONSIDERATIONS

#### 8.1 Potential Difficulties

#### 8.1.1 Runoff Water in Excavation

Water can be expected to collect in the excavation bottom during times of inclement weather or snow melt. To allow observation of the excavation bottom, to reduce the potential for soil disturbance, and to facilitate filling operations, we recommend water be removed from within the excavation during construction.



#### 8.1.2 Disturbance of Soils

The on-site soils can be disturbed under construction traffic, especially if the soils are wet. If soils become disturbed, they should be subcut to the underlying undisturbed soils. The subcut soils can then be dried and recompacted back into place, or they should be removed and replaced with drier imported fill.

#### 8.1.3 Cobbles and Boulders

The alluvium soils at this site can include cobbles and boulders. This may make excavating procedures somewhat more difficult than normal if they are encountered.

### 8.2 Excavation Backsloping

If excavation faces are not retained, the excavations should maintain maximum allowable slopes in accordance with OSHA Regulations (Standards 29 CFR), Part 1926, Subpart P, "Excavations" (can be found on <a href="www.osha.gov">www.osha.gov</a>). Even with the required OSHA sloping, water seepage or surface runoff can potentially induce side slope erosion or sloughing which could require slope maintenance.

### 8.3 Observation and Testing

The recommendations in this report are based on the subsurface conditions found at our test boring locations. Since the soil conditions can be expected to vary away from the soil boring locations, we recommend on-site observation by a geotechnical engineer/technician during construction to evaluate these potential changes. Soil density testing should also be performed on new fill placed in order to document that project specifications for compaction have been satisfied.

## 9.0 ASTM STANDARDS

When we refer to an ASTM Standard in this report, we mean that our services were performed in general accordance with that standard. Compliance with any other standards referenced within the specified standard is neither inferred nor implied.

## 10.0 LIMITATIONS

Within the limitations of scope, budget, and schedule, we have endeavored to provide our services according to generally accepted geotechnical engineering practices at this time and location. Other than this, no warranty, express or implied, is intended.

Important information regarding risk management and proper use of this report is given in Appendix B entitled "Geotechnical Report Limitations and Guidelines for Use."



## **Standard Sheets**

Freezing Weather Effects in Building Floor Slab Moisture Protection Basement Wall Backfill and Water Control



## **Appendix A**

Geotechnical Field Exploration and Testing
Boring Log Notes
Unified Soil Classification System
Figure 1 – Boring Locations
Subsurface Boring Logs
Gradation Curves

# Appendix A Geotechnical Field Exploration and Testing Report No. P-0045582

#### A.1 FIELD EXPLORATION

The subsurface conditions at the site were explored by drilling and sampling 10 standard penetration test borings. The locations of the borings appear on Figure 1, preceding the Subsurface Boring Logs in this appendix.

#### **A.2 SAMPLING METHODS**

#### A.2.1 Split-Spoon Samples (SS)

Standard penetration (split-spoon) samples were collected in general accordance with ASTM: D1586. The ASTM test method consists of driving a 2-inch O.D. split-barrel sampler into the in-situ soil with a 140-pound hammer dropped from a height of 30 inches. The sampler is driven a total of 18 inches into the soil. After an initial set of 6 inches, the number of hammer blows to drive the sampler the final 12 inches is known as the standard penetration resistance or N-value.

#### A.2.2 Disturbed Samples (DS)/Spin-up Samples (SU)

Sample types described as "DS" or "SU" on the boring logs are disturbed samples, which are taken from the flights of the auger. Because the auger disturbs the samples, possible soil layering and contact depths should be considered approximate.

#### A.2.3 Sampling Limitations

Unless actually observed in a sample, contacts between soil layers are estimated based on the spacing of samples and the action of drilling tools. Cobbles, boulders, and other large objects generally cannot be recovered from test borings, and they may be present in the ground even if they are not noted on the boring logs.

Determining the thickness of "topsoil" layers is usually limited, due to variations in topsoil definition, sample recovery, and other factors. Visual-manual description often relies on color for determination, and transitioning changes can account for significant variation in thickness judgment. Accordingly, the topsoil thickness presented on the logs should not be the sole basis for calculating topsoil stripping depths and volumes. If more accurate information is needed relating to thickness and topsoil quality definition, alternate methods of sample retrieval and testing should be employed.

#### **A.3 CLASSIFICATION METHODS**

Soil descriptions shown on the boring logs are based on the Unified Soil Classification (USC) system. The USC system is described in ASTM: D2487 and D2488. Where laboratory classification tests (sieve analysis or Atterberg Limits) have been performed, accurate classifications per ASTM: D2487 are possible. Otherwise, soil descriptions shown on the boring logs are visual-manual judgments. Charts are attached which provide information on the USC system, the descriptive terminology, and the symbols used on the boring logs.

The boring logs include descriptions of apparent geology. The geologic depositional origin of each soil layer is interpreted primarily by observation of the soil samples, which can be limited. Observations of the surrounding topography, vegetation, and development can sometimes aid this judgment.

#### **A.4 WATER LEVEL MEASUREMENTS**

The groundwater level measurements are shown at the bottom of the boring logs. The following information appears under "Water Level Measurements" on the logs:

- Date and Time of measurement
- Sampled Depth: lowest depth of soil sampling at the time of measurement
- Casing Depth: depth to bottom of casing or hollow-stem auger at time of measurement
- Cave-in Depth: depth at which measuring tape stops in the borehole
- Water Level: depth in the borehole where free water is encountered
- Drilling Fluid Level: same as Water Level, except that the liquid in the borehole is drilling fluid

The true location of the water table at the boring locations may be different than the water levels measured in the boreholes. This is possible because there are several factors that can affect the water level measurements in the borehole. Some of these factors include: permeability of each soil layer in profile, presence of perched water, amount of time between water level readings, presence of drilling fluid, weather conditions, and use of borehole casing.

# Appendix A Geotechnical Field Exploration and Testing Report No. P-0045582

#### A.5 LABORATORY TEST METHODS

#### A.5.1 Water Content Tests

Conducted per AET Procedure 01-LAB-010, which is performed in general accordance with ASTM: D2216 and AASHTO: T265.

#### A.5.2 Sieve Analysis of Soils (thru #200 Sieve)

Conducted per AET Procedure 01-LAB-040, which is performed in general conformance with ASTM: D6913, Method

#### A.5.3 Percent Finer than #200 Sieve

Conducted per AET Procedure 01-LAB-060, which is performed in general conformance with ASTM: D1140.

#### A.6 TEST STANDARD LIMITATIONS

Field and laboratory testing is done in general conformance with the described procedures. Compliance with any other standards referenced within the specified standard is neither inferred nor implied.

#### **A.7 SAMPLE STORAGE**

Unless notified to do otherwise, we routinely retain representative samples of the soils recovered from the borings for a period of 30 days.

#### FREEZING WEATHER EFFECTS ON BUILDING CONSTRUCTION

#### **GENERAL**

Because water expands upon freezing and soils contain water, soils which are allowed to freeze will heave and lose density. Upon thawing, these soils will not regain their original strength and density. The extent of heave and density/strength loss depends on the soil type and moisture condition. Heave is greater in soils with higher percentages of fines (silts/clays). High silt content soils are most susceptible, due to their high capillary rise potential which can create ice lenses. Fine grained soils generally heave about 1/4" to 3/8" for each foot of frost penetration. This can translate to 1" to 2" of total frost heave. This total amount can be significantly greater if ice lensing occurs.

#### **DESIGN CONSIDERATIONS**

Clayey and silty soils can be used as perimeter backfill, although the effect of their poor drainage and frost properties should be considered. Basement areas will have special drainage and lateral load requirements which are not discussed here. Frost heave may be critical in doorway areas. Stoops or sidewalks adjacent to doorways could be designed as structural slabs supported on frost footings with void spaces below. With this design, movements may then occur between the structural slab and the adjacent on-grade slabs. Non-frost susceptible sands (with less than 40% by weight passing a #40 sieve and no more than 5% by weight passing a #200 sieve) can be used below such areas. Depending on the function of surrounding areas, the sand layer may need a thickness transition away from the area where movement is critical. With sand placement over slower draining soils, subsurface drainage would be needed for the sand layer. High density extruded polystyrene insulation could be used within the sand to reduce frost penetration, thereby reducing the sand thickness needed. We caution that insulation placed near the surface can increase the potential for ice glazing of the surface.

The possible effects of adfreezing should be considered if clayey or silty soils are used as backfill. Adfreezing occurs when backfill adheres to rough surfaced foundation walls and lifts the wall as it freezes and heaves. This occurrence is most common with masonry block walls, unheated or poorly heated building situations and clay backfill. The potential is also increased where backfill soils are poorly compacted and become saturated. The risk of adfreezing can be decreased by placing a low friction separating layer between the wall and backfill.

Adfreezing can occur on exterior piers (such as deck, fence, or other similar pier footings), even if a smooth surface is provided. This is more likely in poor drainage situations where soils become saturated. Additional footing embedment and/or widened footings below the frost zones (which include tensile reinforcement) can be used to resist uplift forces. Specific designs would require individual analysis.

#### **CONSTRUCTION CONSIDERATIONS**

Foundations, slabs and other improvements which may be affected by frost movements should be insulated from frost penetration during freezing weather. If filling takes place during freezing weather, all frozen soils, snow and ice should be stripped from areas to be filled prior to new fill placement. The new fill should not be allowed to freeze during transit, placement or compaction. This should be considered in the project scheduling, budgeting and quantity estimating. It is usually beneficial to perform cold weather earthwork operations in small areas where grade can be attained quickly rather than working larger areas where a greater amount of frost stripping may be needed. If slab subgrade areas freeze, we recommend the subgrade be thawed prior to floor slab placement. The frost action may also require reworking and recompaction of the thawed subgrade.

#### FLOOR SLAB MOISTURE/VAPOR PROTECTION

Floor slab design relative to moisture/vapor protection should consider the type and location of two elements, a granular layer and a vapor membrane (vapor retarder, water resistant barrier or vapor barrier). In the following sections, the pros and cons of the possible options regarding these elements will be presented, such that you and your specifier can make an engineering decision based on the benefits and costs of the choices.

#### **GRANULAR LAYER**

In American Concrete Institute (ACI) 302.1R-15, a "base material" is recommended over the vapor membrane, rather than the conventional clean "sand cushion" material. The base layer should be a minimum of 4 inches (100 mm) thick, trimmable, compactable, granular fill (not sand), a so-called crusher-run material. Usually graded from  $1\frac{1}{2}$  inches to 2 inches (38 to 50 mm) down to rock dust is suitable. Following compaction, the surface can be choked off with a fine-grade material. We refer you to ACI 302.1R-15 for additional details regarding the requirements for the base material.

In cases where potential static water levels or significant perched water sources appear near or above the floor slab, an under floor drainage system may be needed wherein a draintile system is placed within a thicker clean sand or gravel layer. Such a system should be properly engineered depending on subgrade soil types and rate/head of water inflow.

#### VAPOR MEMBRANE

The need for a vapor membrane depends on whether the floor slab will have a vapor sensitive covering, will have vapor sensitive items stored on the slab, or if the space above the slab will be a humidity controlled area. If the project does not have this vapor sensitivity or moisture control need, placement of a vapor membrane may not be necessary. Your decision will then relate to whether to use the ACI base material or a conventional sand cushion layer. However, if any of the above sensitivity issues apply, placement of a vapor membrane is recommended. Some floor covering systems (adhesives and flooring materials) require installation of a vapor membrane to limit the slab moisture content as a condition of their warranty.

#### VAPOR MEMBRANE/GRANULAR LAYER PLACEMENT

A number of issues should be considered when deciding whether to place the vapor membrane above or below the granular layer. The benefits of placing the slab on a granular layer, with the vapor membrane placed **below** the granular layer, include **reduction** of the following:

- Slab curling during the curing and drying process.
- Time of bleeding, which allows for quicker finishing.
- Vapor membrane puncturing.
- Surface blistering or delamination caused by an extended bleeding period.
- Cracking caused by plastic or drying shrinkage.

The benefits of placing the vapor membrane over the granular layer include the following:

- A lower moisture emission rate is achieved faster.
- Eliminates a potential water reservoir within the granular layer above the membrane.
- Provides a "slip surface", thereby reducing slab restraint and the associated random cracking.

If a membrane is to be used in conjunction with a granular layer, the approach recommended depends on slab usage and the construction schedule. The vapor membrane should be placed above the granular layer when:

- Vapor sensitive floor covering systems are used or vapor sensitive items will be directly placed on the slab.
- The area will be humidity controlled, but the slab will be placed before the building is enclosed and sealed from rain.
- Required by a floor covering manufacturer's system warranty.

The vapor membrane should be placed below the granular layer when:

• Used in humidity controlled areas (without vapor sensitive coverings/stored items), with the roof membrane in place, and the building enclosed to the point where precipitation will not intrude into the slab area. Consideration should be given to slight sloping of the membrane to edges where draintile or other disposal methods can alleviate potential water sources, such as pipe or roof leaks, foundation wall damp proofing failure, fire sprinkler system activation, etc.

There may be cases where membrane placement may have a detrimental effect on the subgrade support system (e.g., expansive soils). In these cases, your decision will need to weigh the cost of subgrade options and the performance risks.

#### BASEMENT/RETAINING WALL BACKFILL AND WATER CONTROL

#### DRAINAGE

Below grade basements should include a perimeter backfill drainage system on the exterior side of the wall. The exception may be where basements lie within free draining sands where water will not perch in the backfill. Drainage systems should consist of perforated or slotted PVC drainage pipes located at the bottom of the backfill trench, lower than the interior floor grade. The drain pipe should be surrounded by properly graded filter rock. A geosynthetic "filter fabric" should then envelope the filter rock. The drain pipe should be connected to a suitable means of disposal, such as a sump basket or a gravity outfall. A storm sewer gravity outfall would be preferred over exterior daylighting, as the latter may freeze during winter. For non-building, exterior retaining walls, weep holes at the base of the wall can be substituted for a drain pipe.

#### **BACKFILLING**

Prior to backfilling, damp/water proofing should be applied on perimeter basement walls. The backfill materials placed against basement walls will exert lateral loadings. To reduce this loading by allowing for drainage, we recommend using free-draining sands for backfill. The zone of sand backfill should extend outward from the wall at least 2 feet, and then upward and outward from the wall at a 30° or greater angle from vertical. The free-draining sand backfill should contain no more than 40% by weight passing the #40 sieve and no greater than 5% by weight passing the #200 sieve. The sand backfill should be placed in lifts and compacted with portable compaction equipment. This compaction should be to the specified levels if slabs or pavements are placed above. Where slab/pavements are not above, we recommend capping the sand backfill with a layer of clayey soil to minimize surface water infiltration. Positive surface drainage away from the building should also be maintained. If surface capping or positive surface drainage cannot be maintained, then the trench should be filled with more permeable soils, such as the Fine Filter or Coarse Filter Aggregates defined in MnDOT Specification 3149. You should recognize that if the backfill soils are not properly compacted, settlements may occur which may affect surface drainage away from the building.

Backfilling with silty or clayey soil is possible but not preferred. These soils can build-up water which increases lateral pressures and results in wet wall conditions and possible water infiltration into the basement. If you elect to place silty or clayey soils as backfill, we recommend you place a prefabricated drainage composite against the wall which is hydraulically connected to a drainage pipe at the base of the backfill trench. High plasticity clays should be avoided as backfill due to their swelling potential.

#### **LATERAL PRESSURES**

Lateral earth pressures on below grade walls vary, depending on backfill soil classification, backfill compaction and slope of the backfill surface. Static or dynamic surcharge loads near the wall will also increase lateral wall pressure. For design, we recommend the following ultimate lateral earth pressure values (given in equivalent fluid pressure values) for a drained soil compacted to 95% of the Standard Proctor density and a level ground surface.

#### **Equivalent Fluid Density**

Soil Type	Active (pcf)	At-Rest (pcf)
Sands (SP or SP-SM)	35	60
Silty Sands (SM)	45	65
Fine Grained Soils (SC, CL or ML)	70	90

Basement walls are normally restrained at the top which restricts movement. In this case, the design lateral pressures should be the "at-rest" pressure situation. Retaining walls which are free to rotate or deflect should be designed using the active case. Lateral earth pressures will be significantly higher than that shown if the backfill soils are not drained and become saturated.

#### DRILLING AND SAMPLING SYMBOLS

Symbol	Definition
ÅR:	Sample of material obtained from cuttings blown out
	the top of the borehole during air rotary procedure.
B, H, N:	Size of flush-joint casing
CAS:	Pipe casing, number indicates nominal diameter in
	inches
COT:	Clean-out tube
DC:	Drive casing; number indicates diameter in inches
DM:	Drilling mud or bentonite slurry
DR:	Driller (initials)
DS:	Disturbed sample from auger flights
DP:	Direct push drilling; a 2.125 inch OD outer casing
	with an inner 1½ inch ID plastic tube is driven
г.	continuously into the ground.
FA:	Flight auger; number indicates outside diameter in
TTA.	inches
HA:	Hand auger; number indicates outside diameter
HSA:	Hollow stem auger; number indicates inside diameter
I.C.	in inches Field logger (initials)
LG: MC:	Column used to describe moisture condition of
MC.	samples and for the ground water level symbols
N (BPF):	Standard penetration resistance (N-value) in blows per
N (DIT).	foot (see notes)
NQ:	NQ wireline core barrel
PQ:	PQ wireline core barrel
RDA:	Rotary drilling with compressed air and roller or drag
1071.	bit.
RDF:	Rotary drilling with drilling fluid and roller or drag bit
REC:	In split-spoon (see notes), direct push and thin-walled
	tube sampling, the recovered length (in inches) of
	sample. In rock coring, the length of core recovered
	(expressed as percent of the total core run). Zero
	indicates no sample recovered.
SS:	Standard split-spoon sampler (steel; 1.5" is inside
	diameter; 2" outside diameter); unless indicated
	otherwise
SU	Spin-up sample from hollow stem auger
TW:	Thin-walled tube; number indicates inside diameter in
	inches
WASH:	Sample of material obtained by screening returning
	rotary drilling fluid or by which has collected inside
W/II	the borehole after "falling" through drilling fluid
WH:	Sampler advanced by static weight of drill rod and
W/D.	hammer
WR:	Sampler advanced by static weight of drill rod 94 millimeter wireline core barrel
94mm: ▼·	Water level directly measured in boring
<u>▼:</u>	water level differry infeasured in borning

Estimated water level based solely on sample

#### **TEST SYMBOLS**

Symbol	Definition
CONS:	One-dimensional consolidation test
DEN:	Dry density, pcf
DST:	Direct shear test
E:	Pressuremeter Modulus, tsf
HYD:	Hydrometer analysis
LL:	Liquid Limit, %
LP:	Pressuremeter Limit Pressure, tsf
OC:	Organic Content, %
PERM:	Coefficient of permeability (K) test; F - Field;
	L - Laboratory
PL:	Plastic Limit, %
$q_p$ :	Pocket Penetrometer strength, tsf (approximate)
q <sub>c</sub> :	Static cone bearing pressure, tsf
q <sub>u</sub> :	Unconfined compressive strength, psf
R:	Electrical Resistivity, ohm-cms
RQD:	Rock Quality Designation of Rock Core, in percent
	(aggregate length of core pieces 4" or more in length
	as a percent of total core run)
SA:	Sieve analysis
TRX:	Triaxial compression test
VSR:	Vane shear strength, remolded (field), psf
VSU:	Vane shear strength, undisturbed (field), psf
WC:	Water content, as percent of dry weight
<b>%-</b> 200:	Percent of material finer than #200 sieve

#### STANDARD PENETRATION TEST NOTES

The standard penetration test consists of driving a split-spoon sampler with a drop hammer counting the number of blows applied in each of three 6" increments of penetration. If the sampler is driven less than 18" (usually in highly resistant material), permitted in ASTM: D1586, the blows for each complete 6" increment and for each partial increment is on the boring log. For partial increments, the number of blows is shown to the nearest 0.1' below the slash.

The length of sample recovered, as shown on the "REC" column, may be greater than the distance indicated in the N column. The disparity is because the N-value is recorded below the initial 6" set (unless partial penetration defined in ASTM: D1586 is encountered) whereas the length of sample recovered is for the entire sampler drive (which may even extend more than 18").

appearance

 $\nabla$ :

#### UNIFIED SOIL CLASSIFICATION SYSTEM ASTM Designations: D 2487, D2488

AMERICAN **ENGINEERING** TESTING, INC.



				Soil Classification	Notes
or Assigning Group Sy	mbols and Group Nar	mes Using Laboratory Tests <sup>A</sup>	Group Symbol	Group Name <sup>B</sup>	ABased on the material passing (75-mm) sieve.
Gravels More than 50% coarse	Clean Gravels Less than 5%	Cu≥4 and 1≤Cc≤3 <sup>E</sup>	GW	Well graded gravel <sup>F</sup>	BIf field sample contained cobble boulders, or both, add "with co
fraction retained	fines <sup>C</sup>	Cu<4 and/or 1>Cc>3 <sup>E</sup>	GP	Poorly graded gravel <sup>F</sup>	boulders, or both" to group name Gravels with 5 to 12% fines re
on ito. I sieve	Gravels with Fines more	Fines classify as ML or MH	GM	Silty gravel <sup>F.G.H</sup>	symbols: GW-GM well-graded gravel
	than 12% fines <sup>C</sup>	Fines classify as CL or CH	GC	Clayey gravel <sup>F.G.H</sup>	GW-GC well-graded gravel GP-GM poorly graded grave
Sands 50% or more of coarse	Clean Sands Less than 5%	$Cu \ge 6$ and $1 \le Cc \le 3^E$	SW	Well-graded sand <sup>I</sup>	GP-GC poorly graded gravel  DSands with 5 to 12% fines requ
fraction passes No. 4 sieve	fines <sup>D</sup>	Cu<6 and/or 1>Cc>3 <sup>E</sup>	SP	Poorly-graded sand <sup>I</sup>	symbols: SW-SM well-graded sand w
	Sands with Fines more	Fines classify as ML or MH	SM	Silty sand <sup>G.H.I</sup>	SW-SC well-graded sand wi SP-SM poorly graded sand v
	than 12% fines D	Fines classify as CL or CH	SC	Clayey sand <sup>G.H.I</sup>	SP-SC poorly graded sand w
Silts and Clays Liquid limit less	inorganic	PI>7 and plots on or above "A" line <sup>J</sup>	CL		
than 50		PI<4 or plots below "A" line <sup>J</sup>	ML	Silt <sup>K.L.M</sup>	$^{E}Cu = D_{60} / D_{10},  Cc = $
	organic	Liquid limit—oven dried <0.75	OL	Organic clay <sup>K,L,M,N</sup>	FIf soil contains >15% sand, add
		Liquid limit – not dried		Organic silt <sup>K.L.M.O</sup>	sand" to group name.  GIf fines classify as CL-ML, use
Silts and Clays Liquid limit 50	inorganic	PI plots on or above "A" line	СН	Fat clay <sup>K.L.M</sup>	symbol GC-GM, or SC-SM.  HIf fines are organic, add "with
or more		PI plots below "A" line	МН	Elastic silt <sup>K.L.M</sup>	fines" to group name.  If soil contains >15% gravel, ac
	organic	Liquid limit-oven dried <0.75	ОН	Organic clay <sup>K,L,M,P</sup>	gravel" to group name.  JIf Atterberg limits plot is hatch
		Liquid limit – not dried		Organic silt <sup>K.L.M.Q</sup>	soil is a CL-ML silty clay.
			PT	Peat <sup>R</sup>	KIf soil contains 15 to 29% plus add "with sand" or "with grave
		in color, and organic in odor			whichever is predominant.
SIEVE ANALYSIS  (m.) Sieve Number  4 10 20 40 60 140 2  Dao = 15mm	RETAINED 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	For dassification of fine-grained soils and fine-grained fraction of coarse-grained soils  50 - Equation of "A"-line Horizontal at PI = 4 to LL = 25.5. then PI = 0.73 (LL-20)  Equation of "U"-line Vertical at LL = 16 to PI = 7. then PI = 0.9 (LL-8)	ort or ort	,r, life	Lif soil contains ≥30% plus No. predominantly sand, add "sa group name.  Mif soil contains ≥30% plus No. predominantly gravel, add "to group name.  NP ≥4 and plots on or above "A OP <4 or plots below "A" line. PPI plots on or above "A" line.
	Gravels More than 50% coarse fraction retained on No. 4 sieve  Sands 50% or more of coarse fraction passes No. 4 sieve  Silts and Clays Liquid limit less than 50  Silts and Clays Liquid limit 50 or more	Gravels More than 50% coarse fraction retained on No. 4 sieve  Sands 50% or more of coarse fraction passes No. 4 sieve  Silts and Clays Liquid limit less than 50  Silts and Clays Liquid limit so or ganic  Silts and Clays Liquid limit 50 or more  Silts and Clays Liquid limit 50 or more	than 50% coarse fraction retained on No. 4 sieve    Cu<4 and/or 1>Cc>3E	Group Symbols and Group Names Using Laboratory Tests^A       Group Symbol         Gravels More than 50% coarse fraction retained on No. 4 sieve       Clean Gravels Less than 5% finesc²       Cu≥4 and 1≤Cc≤3E       GW         Group Symbol         Gravels More than 50% coarse fraction retained on No. 4 sieve       Less than 5% fines classify as ML or MH       GM         Sands 50% or more of coarse fraction passes No. 4 sieve       Clean Sands Less than 5% finesb photograph       Cu≥6 and 1≤Cc≤3E       SW         Sands with Fines classify as ML or MH       SM         Fines classify as ML or MH	Gravels More than 50% coarse fraction retained on No. 4 sieve    Gravels with Fines more than 12% fines C   Cu≤4 and 1≤Cc≤3E   GW   Well graded gravelF

ABased on the material passing the 3-in
(75-mm) sieve.
Brc @ 11 1 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

ined cobbles or d "with cobbles or group name. % fines require dual

led gravel with silt ed gravel with clay ded gravel with silt ded gravel with clay fines require dual

ed sand with silt d sand with clay led sand with silt ed sand with clay

 $(D_{30})^2$  $D_{10}\,x\;D_{60}$ 

sand, add "with

L-ML, use dual C-SM.

add "with organic gravel, add "with

ot is hatched area,

clay.

29% plus No. 200 vith gravel", nant.

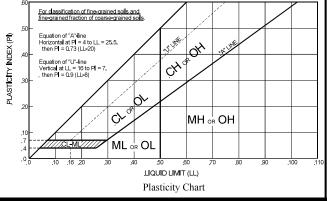
plus No. 200, d, add "sandy" to

6 plus No. 200, vel, add "gravelly"

above "A" line.

QPI plots below "A" line.

<sup>R</sup>Fiber Content description shown below.



	ADDITIONAL TERM	INOLOGY NO	OTES USED BY AE	T FOR SOIL ID	DENTIFICATION ANI	D DESCRIPTION	
<u>Term</u>	Grain Size Particle Size	<u>Grave</u> <u>Term</u>	l <u>Percentages</u> <u>Percent</u>	Consistend Term	cy of Plastic Soils N-Value, BPF	Relative Densit Term	y of Non-Plastic Soils N-Value, BPF
Boulders Cobbles Gravel Sand Fines (silt & clay	Over 12" 3" to 12" #4 sieve to 3" #200 to #4 sieve  Pass #200 sieve	A Little Grav With Gravel Gravelly	el 3% - 14% 15% - 29% 30% - 50%	Very Soft Soft Firm Stiff Very Stiff Hard	less than 2 2 - 4 5 - 8 9 - 15 16 - 30 Greater than 30	Very Loose Loose Medium Dense Dense Very Dense	0 - 4 5 - 10 11 - 30 31 - 50 Greater than 50
D (Dry):  M (Moist):  W (Wet/ Waterbearing):	sture/Frost Condition (MC Column) Absence of moisture, dusty, dry to touch. Damp, although free water not visible. Soil may still have a high water content (over "optimum"). Free water visible, intended to describe non-plastic soils. Waterbearing usually relates to sands and sand with silt. Soil frozen		Layers less than ½" thick of differing material or color.  Pockets or layers greater than ½" thick of differing material or color.	Peat  Term  Fibric Peat: Hemic Peat: Sapric Peat:	Fiber Content (Visual Estimate)  Greater than 67% 33 – 67% Less than 33%	Soils are described a and is judged to ha content to influence Slightly organic used Root Ir With roots: Judged of root proper Trace roots: Small I to be in	ription (if no lab tests) is organic, if soil is not peat ave sufficient organic fines the Liquid Limit properties. If or borderline cases, inclusions to have sufficient quantity is to influence the soil ties. The coots present, but not judged a sufficient quantity to cantly affect soil properties.



AET JO							LO	G OF	BOI	RING N	O	В	-01 (	p. 1 o	f 1)	
PROJEC	004	developm				175					Λ	2 277	402			
SURFAC	CE ELEVATION: <b>884</b>		LATITUD	E:	44.949	1/5		LON	NGIT	ΓUDE:	-9	3.277				
DEPTH IN FEET	MATERIAL I	DESCRIPTIO	N		GEOI	LOGY	N	MC	SA T	MPLE YPE	REC IN.	WC	DEN	BORA		TEST
1 -	3½' Bituminous pavement 6" FILL, silty sand with gr		n (apparent		FILL		12	M	M	SS	16					
2 - 3 - 4 -	base) FILL, mostly silty sand wark brown	ith gravel, t	prown and	]			12	M	X	SS	18					13
5 — 6 —							4	M		SS	16					
7 — 8 — 9 —	GRAVELLY SAND, fine brown, moist, loose (SP) (	possible fil	1)		COAR ALLU OR FII	VIUM	6	M	X	SS	14					
10 — 11 —	SAND, a little gravel, fine light brown, moist, medium	to medium m dense (S	grained, P)		COAR ALLU		12	M		SS	18					
12 — 13 — 14 —							13	M	X	SS	16					
15 — 16 — 17 —	SAND, fine to medium gr moist, medium dense (SP)	ained, light	brown,				25	M	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	18					
18 — 19 — 20 — 21 — 22 —	SAND, a little gravel, fine light brown, moist, medium	to medium m dense (S	grained, P)				17	М	17-13-X-P3	SS	20					
23 — 24 — 25 — 26 — 27 —	SAND, fine grained, light very dense (SP)	brown, mo	ist, dense to	<b>D</b>			43	M	111111111111111111111111111111111111111	SS	24					
28 - 29 - 30 - 31 - 32 - 33 -							51	М	17-71   \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\	SS	24					
34 - 35 - 36 -							47	M	{{\}	SS	24					
	END OF BORING			1												
DEP	TH: DRILLING METHOD			WAT	ER LEV	EL MEA	SURF	EMEN	TS		<u> </u>		Η,	NOTE:	DEEL	р т
0-34		DATE	TIME	SAMPI DEPT		ASING EPTH		/E <b>-</b> IN PTH	Г	ORILLIN UID LE	NG VEL	WATI LEVE		NOTE: THE A		
0-3-	1/2 JOHN HUM	9/5/25	10:45	36.5		34.5		5.5			$\dashv$	Non		SHEET	rs foi	R AN
		9/5/25	10:55	36.5	_	34.5		5.4			+	Non	┥,	EXPLA	NATI(	ON O
BORIN	G 0/5/25	713123	10.33	50		<b></b>	30	<b>,,</b> ,,				14011		ERMIN	OLOG	GY (
	LETED: 9/5/25										+		-		IS LO	
DR: <b>J</b> (	C LG: <b>GH</b> Rig: <b>110</b>														01 <b>-</b> DF	



AET JC	P-004558	2				LC	G OF	BOI	RING N	О.	В	-02 (	p. 1 o	f 1)	
PROJEC		edevelopm	*		*						2 2 = =	100			
	CE ELEVATION:882		LATITUE	ÞΕ:	44.949030		LO	NGI'	ΓUDE:	-9	3.277				
DEPTH IN FEET	MATERIAL	DESCRIPTIO	ON		GEOLOGY	N	MC	SA	MPLE YPE	REC IN.		1	BORA		T
FEET	√3" Bituminous pavement				FILL			1		11	WC	DEN	LL	PL	<b>%</b> -#.
1 -	FILL, mostly sand with s	ilt, a little cl	ayey sand	-/	TILL	18	M	X	SS	14					
2 —	with gravel, pieces of bitu					10		M	G G	20					
3 — 4 —						19	M	$\square$	SS	20					
5 —	FILL, mixture of sand an	d silty sand	, a little			_		M	CC	16					
6 —	gravel, light brown and br	rown				5	M	Д	SS	16					
7 —	SAND, a little gravel, fine	e to medium	grained,		COARSE			M	CC	1.0					
8 — 9 —	light brown, moist, loose	(SP)			ALLUVIUM	9	M	Д	SS	18					
10 —	SAND WITH GRAVEL,					10	M	M	SS	1.0					
11 -	grained, light brown, moi laminations of clayey san	st, medium ( d (SP)	dense,			18	M	Д	သ	18					
12 — 13 —	SAND, a little gravel, find	e to medium	grained,			19	N	M	SS	20					
13 – 14 –	light brown, moist, medic laminations of sand with	ım dense, le silt (SP)	nses and			19	M	A	သ	20					
15 —		, ,				21	M	M	SS	20					
16 —						21	IVI	A	33	20					
17 — 18 —								$\mathbb{R}$							
19 <b>–</b>	SAND, mostly fine grain (SP)	ed, brown, n	noist, dens	e 🗀											
20 —						32	M	M	SS	22					
21 —	END OF BORING					32	IVI	И		22					
DEF	TH: DRILLING METHOD			1	ER LEVEL MI			TS				1	NOTE:	REFE	R TO
0-1	9½' 3.25" HSA	DATE	TIME	SAMPI DEPT	ED CASING H DEPTH	6 CAV	/E <b>-</b> IN PTH		ORILLIN UID LE		WATI LEVE		THE A	TTAC	HED
	UND ARMAR	9/3/25	1:55	21.5	5 19.5	2	1.5				Non	e	SHEET	S FOI	R AN
		9/3/25	2:05	21.5	5 19.5	2	1.4				Non	e E	EXPLA	NATIO	ON C
BORIN COMP	IG LETED: <b>9/3/25</b>											T	ERMIN	OLOG	GY (
DR: J													TH	IS LO	G

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PROJEC	905	developm	-								2 25/	071			
SURFAC	EE ELEVATION: 885		LATITUD	E:4	4.949176		LO	NGI'	ΓUDE:	9	3.276				
DEPTH IN FEET	MATERIAL I	DESCRIPTIO	ON		GEOLOGY	N	MC	SA T	MPLE YPE	REC IN.	WC	D& LA DEN	BORAT	FORY PL	TEST %-#2
1 - 2 -	3" Bituminous pavement FILL, mostly sand, a little	gravel, pie	ces of		FILL	8	М	M	SS	16					
$\begin{bmatrix} 2 \\ 3 \\ 4 \end{bmatrix}$	bituminous, dark brown FILL, mixture of sand and gravel, brown and dark bro		a little	-1		12	M	M	SS	16					
5 - 6 -	FILL, mixture of gravelly sand, dark brown SAND, mostly fine grainer	silty sand a			COARSE ALLUVIUM	9	M	X	SS	18					
7 <del>-</del> 8 <del>-</del> 9 <del>-</del>	loose, laminations of sand SAND, a little gravel, fine light brown, moist, mediur	with silt (S to medium	SP) grained,	<b>√</b>		16	M	PA A	SS	20					
10 -	of silty sand (SP) GRAVELLY SAND, med light brown to brown, moi	ium to fine	grained,	_		17	M	X	SS	20					
12 — 13 — 14 —	dense (SP)					9	M		SS	18					
15 — 16 — 17 —	SAND, a little gravel, fine light brown, moist, mediur (SP)	to medium n dense to	n grained, very dense			21	M	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	22					
18 — 19 — 20 — 21 — 22 —						48	М		SS	22					
23 – 24 – 25 – 26 – 27 –						54	М	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	24					
28 — 29 — 30 — 31 — 32 — 33 —	SAND, mostly fine grained lenses of sand with silt (SF	d, light bro	wn, dense,			50	М	111 \ P11	SS	24					
34 – 35 – 36 –						33	M	<u>}</u>	SS	22					
	END OF BORING			11. 1											
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1 – 2 –	3" Bituminous pavement 6" FILL, silty sand with grass)	•	` * *		FILL		19	M	M	SS	8					
3 — 4 — 5 —	FILL, a mixture of sand, s sand and sandy lean clay, a piece of concrete, brown a	a little grav	el and a				13	M M	X P	SS SS	18					
6 — 7 — 8 —	GRAVELLY SAND, fine light brown, moist, medium	to medium	grained,		: COARSE		16	M	R	SS	18					
9 – 10 – 11 –	ing. to every more, meaning	(8	- )				29	M	PA X	SS	16					
12 — 13 — 14 —	SAND, a little gravel, fine light brown, moist, medium	to medium n dense to	n grained, dense (SP)				21	М	TA A	SS	20					
15 — 16 — 17 — 18 —							15	M	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	20					
19 – 20 – 21 –	END OF BORING						32	M	{{ }	SS	22					
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0-19	9½' 3.25" HSA	DATE	TIME	SAMPI DEP	LED CASI	ING TH	CAV DEI	E-IN PTH	FL	ORILLIN UID LE	NG VEL	WATI LEVE	ER EL	ТНЕ А	TTAC	CHEI
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1 -	3" Bituminous pavement 5" FILL, silty sand with gr	avel, brow	n (apparent		FILL	17	M	M	SS	22					
2 – 3 –	base) FILL, a mixture of silty sai	nd and clay		]		27	M	M	SS	20					
4 – 5 –	with gravel, brown and dar SAND, a little gravel, fine	to medium	grained,		COARSE	4									
6 <del>-</del> 7 <del>-</del>	light brown, moist, mediur	n dense (SI	P)		ALLUVIUM	15	M	N P	SS	22					
8 - 9 -						12	M		SS	22					
10 -	SAND WITH GRAVEL, f	ine to med	ium dense (SP)			12	M	M	SS	18					
12 —						1.6		13	G.G.	1,4					
13 – 14 –	GAND G					16	M	A P	SS	14					
15 — 16 —	SAND, fine to medium gramoist, medium dense (SP)	uned, light	brown,			15	M	M	SS	22					
17 — 18 —								<b>}</b>							
19 —								团							
20 —						21	M	M	SS	20					
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0-19	9½' 3.25" HSA	DATE 9/4/25 9/4/25	9:15 9:25	WATI SAMPI DEPT 21.5 21.5	5 19.5	CAV DE	EMEN' /E-IN PTH 1.5	П	DRILLIN UID LE	NG VEL	WATI LEVE Non	ER EL e e	NOTE: THE A SHEET	TTAC IS FOI NATIO	CHEC R AN ON C
BORIN COMPL	ETED: <b>9/4/25</b>											T	ERMIN	1OLO	ĴΥ (
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1 -	3" Bituminous pavement 6" FILL, silty sand with g	ravel, brow	n (apparent		FILL	15	М	M	SS	16					
2 3 4	\base) \text{FILL, a mixture of silty sa} \little gravel, brown and da}	nd and clay	vey sand, a		COARSE	18	M	M	SS	20					
5 — 6 —	SAND WITH GRAVEL, grained, light brown, mois	fine to med	ium dense (SP)		ALLUVIUM	18	M		SS	18					
7 — 8 —						18	M	F	SS	16					
9 — 10 —															
11 — 12 —	SAND, a little gravel, fine	to madi	omains d			27	M	X P	SS	20					
13 — 14 —	light brown, moist, medium	m dense (S	i grained, P)			21	M	X	SS	20					
15 — 16 —	SAND, fine grained, light dense to dense (SP)	brown, mo	ist, medium	1		25	M		SS	22					
17 — 18 —								{} {{							
19 —								【】							
20 — 21 —						39	M	X	SS	22					
DEP <b>0-1</b> 9		DATE	TIME	SAMPI DEPT		CAV DE	/E <b>-</b> IN PTH	1	ORILLIN UID LE	NG VEL	WATE	ER EL	NOTE: THE A	TTAC	HEI
		9/4/25	12:03	21.5			1.5			-+	Non		XPLA		
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PROJEC	-	developme	ent; Minno		-						2 2 5 5	<b>(5)</b>			
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DEPTH IN FEET	MATERIAL I	DESCRIPTIO	N		GEOLOGY	N	MC	SA T	MPLE YPE	REC IN.	WC	D& LA DEN	BORAT	I	TEST %-#2
1 -	4" Bituminous pavement 6" FILL, silty sand with gr	ravel, browi	n (apparent	/4	FILL	12	M	M	SS	16					
3 -	base) FILL, mostly silty sand, a little gravel, brown	little clayey	sand, a			17	M	M	SS	18					2
4 – 5 – 6 –	FILL, mostly sand, light by brown and black	rown, a littl	e dark			17	M	F	SS	22					
7 – 8 –						19	M		SS	20					
9 –	FILL, mostly sand with sil	t with grave	el, brown			20	M		SS	14					
11 - 12 - 13 -	SAND WITH GRAVEL,	fine to medi	ium		COARSE ALLUVIUM	22	M		SS	18					
14 – 15 –	grained, light brown, mois SAND, fine to medium gra		. ,		ALLUVIUM										
16 – 17 –	moist, medium dense (SP)	, 5	,			15	M	X {}	SS	20					
18 — 19 — 20 — 21 — 22 —	SAND, a little gravel, fine light brown, moist, medium	to medium m dense to o	grained, dense (SP)			17	M	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	22					
23 - 24 - 25 - 26 - 27 -						26	М	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	SS	20					
28 - 29 - 30 - 31 - 32 -						42	M		SS	24					
33 – 34 – 35 – 36 –						45	М	<u>{</u>	SS	18					
Ī	END OF BORING							П							
DEP	ГН: DRILLING METHOD			WATE	R LEVEL ME	1 ASURF	L EMEN'	L ΓS				<u> </u>	LOTE	DEET	'D 7
		DATE	TIME	SAMPL DEPT		1	/E <b>-</b> IN PTH	Г	RILLIN UID LE	NG VEI	WATI LEVE		NOTE: THE A		
0-34	1½' 3.25" HSA	9/5/25						11	טוט טוט	4 LL		_	SHEET	S FOI	R AN
			9:23	36.5			6.4				Non	<u> </u>	XPLA		
BORING	G	9/5/25	9:33	36.5	34.5	30	6.2			+	Non		ERMIN		
COMPL	ETED: 9/5/25					-						1			
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	FILL, mostly silty sand, a	little olovov	y cand a	т.	·ILL			1	пь	11N.	WC	DEN	LL	PL	<b>%</b> -#2
1 - 1	ittle gravel, trace roots, br	own and da	y sanu, a ark brown		ILL	14	M	X	SS	20					
$\frac{2}{2}$						12	M	M	SS	18					
3 - 4 -						12	M	A	33	10					
5 - F	FILL, mostly sand, a little	silty sand a	and sandy			24	M	M	SS	20					
o ∣ t	ean clay, a little gravel, br brown	own, a littl	le dark			24	IVI	A	33	20					
7 — 8 —						15	M	M	SS	18					
9 –						13	IVI	A	33	10					
10 -						20	M	M	SS	18					
11 -						20	141		SS	10					
12 <del>-</del> 13 <del>-</del>						39	M	M	SS	18					
14 –						39	141		SS	10					
15 – S	SAND, fine grained, light	brown, mo	ist, medium		COARSE ALLUVIUM	22	M	M	SS	20					
10	lense to dense (SP)				ALLU VIUWI	22	IVI		55	20					
17 — 18 —								${}$							
19 –								${}$							
20 -						41	M	M	SS	24					
21 -						71	IVI		33	24					
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2	SAND, fine to medium grandst, medium dense (SP)	ained, light	brown,					M							
25	mensa, mearann aense (er)					27	M	M	SS	20					
26 —						2'	171		55	20					
27 –								${}$							
2	SAND, a little gravel, fine ight brown, moist, dense (	to medium	n grained,					X							
30 -	ight brown, moist, dense (	(51)				45	M	M	SS	22					
31 —						43	IVI	Д	33						
32 -								${}$							
5	SAND, fine grained, light SP)	brown, mo	ist, dense					X							
35 —	51)					44	M	M	SS	24					
36 —	IND OF BODING						171	И							
I	END OF BORING														
DEPTH	I: DRILLING METHOD				R LEVEL MEA								NOTE:	REFE	ER TO
0-341/2	2' 3.25" HSA	DATE	TIME	SAMPLE DEPTH	D CASING DEPTH	CAV	/E <b>-</b> IN PTH	FL	ORILLIN UID LE	VEL	WATI LEVE	ER   L	THE A	TTAC	HED
		9/4/25	10:44	36.5	34.5	36	5.5				Non	e	SHEET	rs foi	R AN
		9/4/25	10:54	36.5	34.5	36	5.3				Non		EXPLA		
BORING COMPLE	ГЕD: <b>9/4/25</b>											T	ERMIN		
DR: <b>JC</b>	LG: <b>GH</b> Rig: <b>110</b>												TH	IS LO	G



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DEI II FE	PTH N ET		N	IATERIAL I	DESCRIPTIO	)N		GE	EOLOGY	N	MC	SA	MPLE YPE	REC IN.	WC		BORAT	PL	FESTS %-#200
	1 -	FILL little	, mostly sa gravel, bro	nd, a little wn and da	sandy lean	clay, a		FIL	L	16	M	M	SS	20					
	2 - 3 -		8							16	M	$\bigvee$	SS	18					
	4 +	SAN	D WITH C	RAVEL,	fine to med	ium		CO	ARSE			$\bigvee$							
	5 - 6 -	-	ed, brown of BORI	` ′				ALI	LUVIUM	17	M	И	SS	20					
AET_CORP W4.AT-LONG P-0045582 NEW NICOLLET REDEVELOPMENT MINNEAPOLIS MN.GPJ AET+CPT+WELL.GDT 10/20/25  C O C C C C C C C C C C C C C C C C C																			
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AT-LO.	or viac next				9/5/25	11:20	6.0		4.0	5	.9				Non			ΓS FOR	
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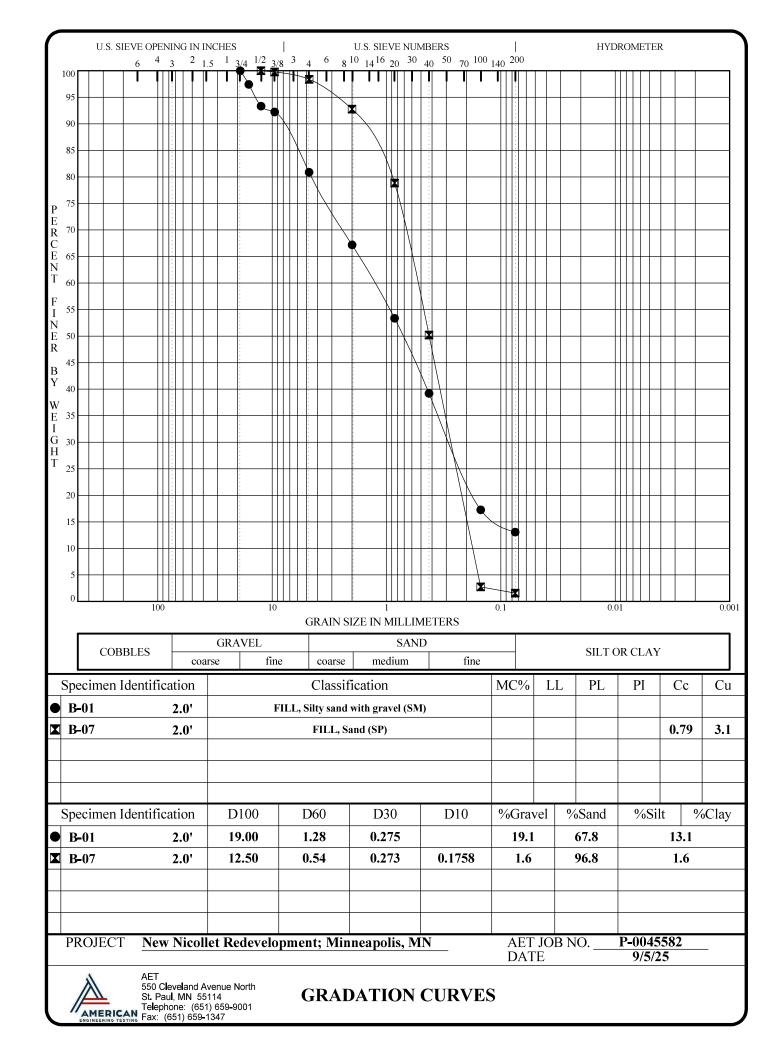
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I D	EPTH IN FEET		MATERIAL I	DESCRIPTIC	ON		GE	OLOGY	N	MC	SA	MPLE FYPE	REC IN.		DEN	LL		% <del>-</del> #200
	1 - 2 - 3 -	FILL grave brow	, a mixture of sand an el and a piece of coal, n	d silty sand brown and	d with dark		FILL	J	11	M M	X	SS SS						
	4 — 5 —	SAN grain	D WITH GRAVEL, fled, light brown, mois	fine to med t, medium	ium dense (SP)			ARSE UVIUM	18	M	M	SS	22					
CORP W-LAT-LONG P-0045582 NEW NICOLLET REDEVELOPMENT MINNEAPOLIS MN.GPJ AET+CPT+WELL.GDT 10/20/25	6 —	END	OF BORING															
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Į.				713143	11.43	0.0		7.0	3	.0				14011		EXPLA		
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## **Appendix B**

Geotechnical Report Limitations and Guidelines for Use

# Appendix B Geotechnical Report Limitations and Guidelines for Use Report No. P-0045582

#### **B.1 REFERENCE**

This appendix provides information to help you manage your risks relating to subsurface problems which are caused by construction delays, cost overruns, claims, and disputes. This information was developed and provided by GBA<sup>1</sup>, of which, we are a member firm.

#### **B.2 RISK MANAGEMENT INFORMATION**

#### B.2.1 Understand the Geotechnical Engineering Services Provided for this Report

Geotechnical engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical engineering services is typically a geotechnical engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

## **B.2.2** Geotechnical Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical engineering study is unique, each geotechnical engineering report is unique, prepared solely for the client.

Likewise, geotechnical engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. If you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.

<sup>1</sup> Geoprofessional Business Association, 15800 Crabbs Branch Way, Suite 300, Rockville, MD 20855 Telephone: 301/565-2733: www.geoprofessional.org, 2019

## Appendix B Geotechnical Report Limitations and Guidelines for Use Report No. P-0045582

#### **B.2.3** Read the Full Report

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. Read and refer to the report in full.

#### **B.2.4** You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- · the composition of the design team; or
- · project ownership.

As a general rule, always inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

#### B.2.5 Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface using various sampling and testing procedures. Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed. The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

#### B.2.6 This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report — including any options or alternatives — are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations only after observing actual subsurface conditions exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.

#### **B.2.7 This Report Could Be Misinterpreted**

Other design professionals' misinterpretation of geotechnical engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- · confer with other design-team members;
- help develop specifications;
- · review pertinent elements of other design professionals' plans and specifications; and
- be available whenever geotechnical engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

#### **B.2.8 Give Constructors a Complete Report and Guidance**

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical engineering report, along with any attachments or appendices, with your contract documents, but be certain to note conspicuously that you've included the material for information purposes only. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the

# Appendix B Geotechnical Report Limitations and Guidelines for Use Report No. P-0045582

report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, only from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and be sure to allow enough time to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

#### **B.2.9 Read Responsibility Provisions Closely**

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. Read these provisions closely. Ask questions. Your geotechnical engineer should respond fully and frankly.

#### **B.2.10 Geoenvironmental Concerns Are Not Covered**

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical engineering study. For that reason, a geotechnical engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. Unanticipated subsurface environmental problems have led to project failures. If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

#### B.2.11 Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer's services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. Geotechnical engineers are not building-envelope or mold specialists.